

### Second-Order Analysis of an Uncracked Member

1. ECCENTRICITY OF APPLIED LOAD =	8	in.
2. TEMP. DIFF. INSIDE TO OUTSIDE =	50	Degrees
3. MIDSPAN BOWING DUE TO TEMP. =	0.61	in.
4. BRACED FRAME - NO JOINT TRANSLATION		
5. JOINTS ASSUMED PINNED TOP AND BOTTOM.		
6. MEMBER HAS A SIMPLE SPAN LENGTH =	32	ft.
7. MOMENTS OR DEFLECTIONS THAT CAUSE OUTWARD BOWING ARE POSITIVE.		
8. SERVICE LOAD:		
LATERAL LOADS DUE TO WIND, W =	0.2	kip/ft.
APPLIED DEAD LOAD, P(dl) =	10	kips
APPLIED LIVE LOAD, P(ll) =	10	kips
PANEL WEIGHT AT MID-HEIGHT =	12.0	kips

OUTWARD

THERE'S A THIN WALL BETWEEN SUSTAINABILITY AND STRENGTH.

# THiN-Wall

*It's all in one.*

Design Guide Brochure

### Section Properties:

Cracking Moment Modification Factor, CMMF =		.075
I <sub>gross</sub> =	7020	in <sup>4</sup>
S(in) =	1560	in <sup>3</sup>
S(out) =	1560	in <sup>3</sup>
AREA =	720	in
f'c =	6	ksi
Concrete Unit Weight	150	pcf
E <sub>c</sub> =	4696	ksi

### Pre Stressing Data:

F <sub>pu</sub> =	270	ksi	PRESTRESS LOSSES =	0.1	Decimal
Total A <sub>ps</sub> =	0.85	in	PRESTRESS ECCEN =	0	in.
JACKING =	0.7	Decimal	(ECCEN. IS + IF BOWING IS OUTWARD)		

### Loading Assumption #1 : U = 1.2D + 1.6W + 0.5L

Truss Beam Theory Modification Factor, TBMF =		0.25
W <sub>u</sub> =	0.320	kip/ft.
BETA(d) +	0.71	
P <sub>u</sub> =	17.0	kips
P <sub>u</sub> (dl) =	12.0	kips
PHI (k) =	0.75	
EI = TBMF * PHI(k) * E <sub>c</sub> * I <sub>g</sub> / (1 + BETA(d)) =	3.62E+06	kip-in <sup>2</sup>

### Pre Stressing Data:

DEFLECTION AT MIDSPAN DUE TO P <sub>u</sub> *e =	0.346	in.
DEFLECTION AT MIDSPAN DUE TO P/S =	0.000	in.
DEFLECTION AT MIDSPAN DUE TO W <sub>u</sub> =	1.221	in.
TOT. ULT. DEFLECTION AT MIDSPAN + TEMP. =	2.182	in.

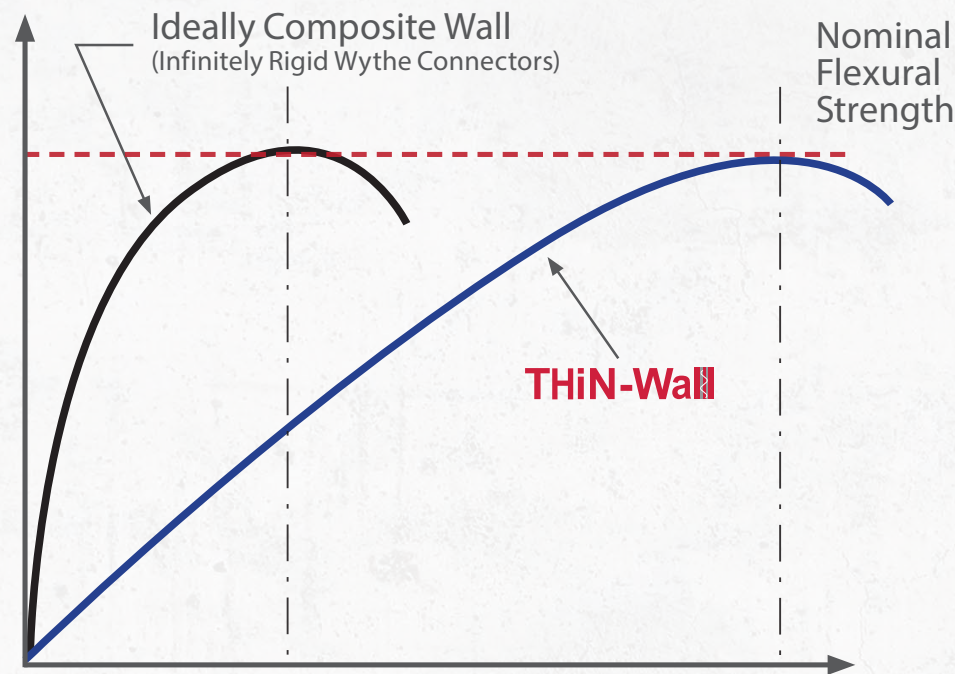
### Determine Additional Deflection at Midspan Due to Total Ultimate Deflection

DELTA1 =	0.189	in.	DELTA2 =	0.205	in.
DELTA3 =	0.206	in.	DELTA4 =	0.207	in.
DELTA5 =	0.207	in.	DELTA6 =	0.207	in.

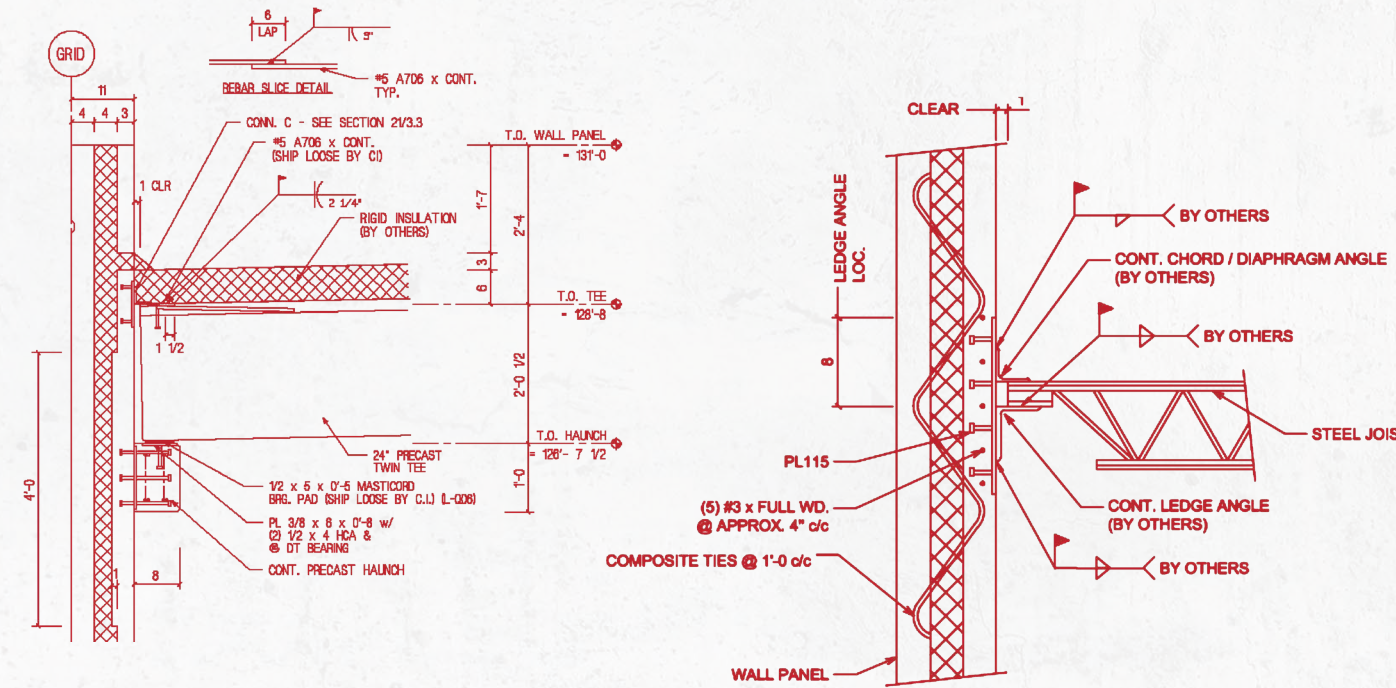
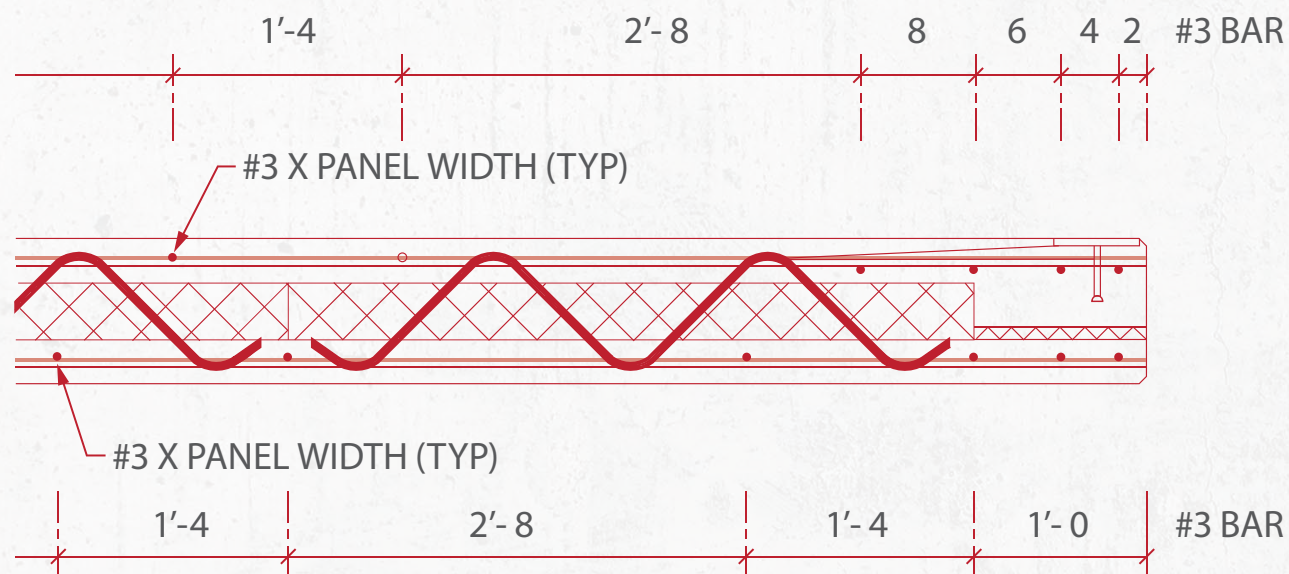


## THiN-Wall —Redefining what eco-friendly buildings can be.

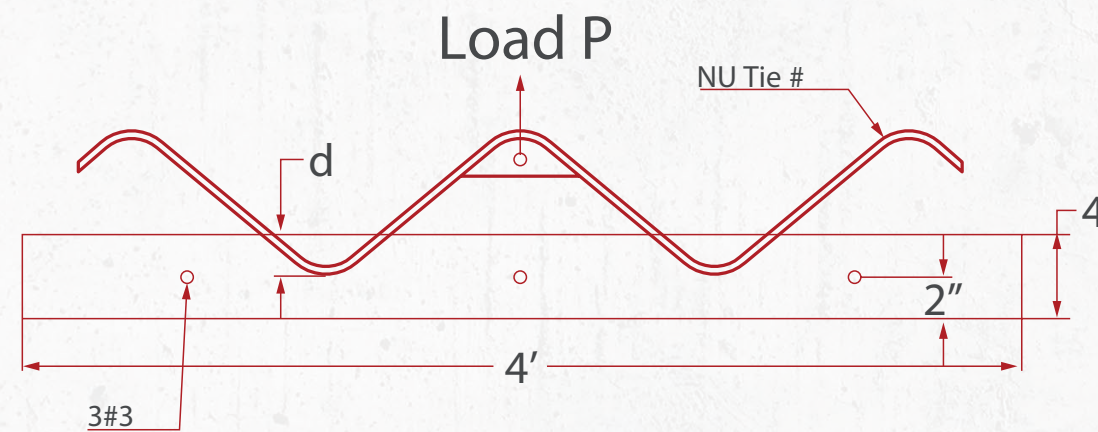
With two 3" layers of concrete, THiN-Wall delivers the same insulating and load-bearing performance of thicker walls while using less cement, which minimizes carbon dioxide emissions. THiN-Wall has architects rethinking their approach to sustainable buildings.



- 100% Composite Action for "Nominal Flexural Strength"
- 75% Composite Action for "Flexure Cracking Checks"
- 25% Composite Action for "Deflection Analysis"



NU-Tie Size	Embedment Depth (d)	Ultimate Load (P) in pounds					
		Test #1	Test #2	Test #3	Test #4	Test #5	Test #6
#3	0.175	525	623	479	Pull-out	542	0.14
	1	1,594	906	1,431	Pull-out	1,310	0.27
	1.5	3,091	3,534	1,686	Pull-out	2,770	0.35
	2	6,145	6,387	5,565	Tie rupture	6,032	0.07

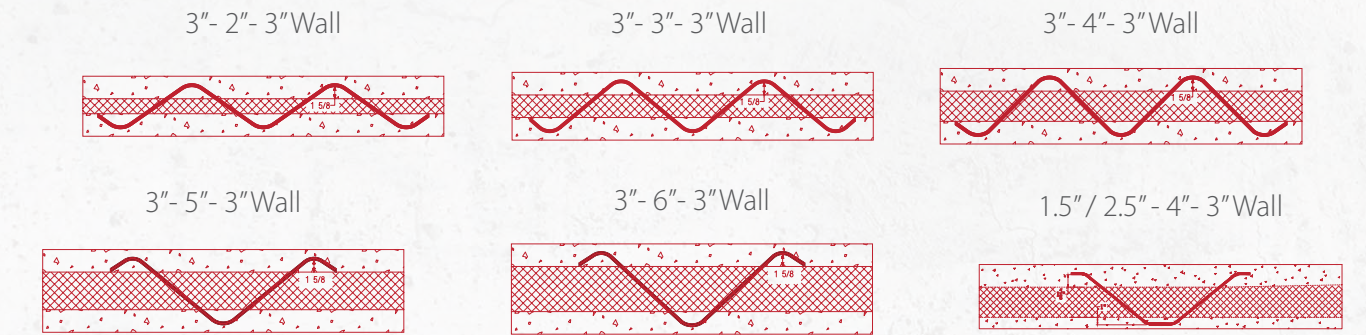


### Contact Information

To learn more, visit [thin-wall.com](http://thin-wall.com) or email [Douglas.Gremel@owenscorning.com](mailto:Douglas.Gremel@owenscorning.com) to request a USB drive full of additional information.

## Nu-Tie Shear Connector

- Very High Tensile Strength - 110 KSI
- Thermally Non-Conductive
- Low Stiffness (Modulus) Mitigates Thermal Bowing - 7,000 KSI
- Strong yet Flexible is the best Balance of Strength and Stiffness for Wall Panel Construction
- 2" Embedment - 3 kip pullout capacity



### Benefits of Nu-Tie

- Fully Insulated Wall Panels with NO thermal bridges from connectors
- Use less concrete - Overall wythe thickness reduced
- Use the insulation of your choice from multiple suppliers (EPS or XPS)
- Straight forward and validated design methodology

### Design Guidance

The design of structural load bearings walls for the THiN-Wall system follows the methodology of the PCI Handbook. The Nu-Ties are oriented vertically and positioned as per the latest version of the THiN-Wall design software.

Part Number	Insulation Thickness	Color Code
RNU3 - 216H	2" (50mm)	Green
RHU3 - 317H	3" (75mm)	Red
RHU3 - 418H	4" (100mm)	Yellow
RNU3 - 519H - 24P	5" (125 mm)	Blue
RNU3 - 6110H - 24P	6" (150 mm)	White
RNU2 - 417H - 24FP	4" (100 mm)	Red